# Statically Indeterminate Frame Example Steven Vukazich <br> San Jose State University 

## Steps in Solving an Indeterminate Structure using the Force Method

Determine degree of Indeterminacy
Let $n=$ degree of indeterminacy
(i.e. the structure is indeterminate to the nth degree)

Define Primary Structure and the $n$ Redundants


## Example Problem



For the indeterminate frame subjected to the point loads shown, find the support reactions and draw the bending moment diagram for the frame. $E I$ is the same for both the horizontal and vertical members.

## FBD of the Frame



## Define Primary Structure and Redundant

- Remove all applied loads from the actual structure;
- Remove support reactions or internal forces to define a primary structure;
- Removed reactions or internal forces are called redundants;
- Same number of redundants as degree of indeterminacy
- Primary structure must be stable and statically determinate;
- Primary structure is not unique - there are several choices.



## Define and Solve the Primary Problem

- Apply all loads on actual structure to the primary structure;
- Define a reference coordinate system;
- Calculate relevant deflections at points where redundants were removed.



## Solve the Primary Problem



## Solve the Primary Problem

## Virtual System <br> to measure $\Delta_{D}$

B


Need to
construct
the $M_{Q}$
diagram

## FBD of the Primary Problem


$\mathrm{M}_{\mathrm{P}}$ Diagram for the Primary Problem


FBD of the Virtual System

$\mathrm{M}_{\mathrm{Q}}$ Diagram for the Primary Problem


## Solve the Primary Problem

$$
1 \cdot \Delta_{D}=\frac{1}{E I} \int_{0}^{L} M_{Q} M_{P} d x
$$

$$
\Delta_{D}=-\frac{23,125 \mathrm{k}-\mathrm{ft}^{3}}{E I}
$$



250 k-ft

$$
-\frac{1}{2} M_{1}\left(M_{3}+M_{4}\right) L \quad-\frac{1}{6}\left(M_{1}+2 M_{2}\right) M_{3} L
$$

$-\left(\frac{1}{2}\right)(10 \mathrm{ft})(250 \mathrm{k}-\mathrm{ft}+150 \mathrm{k}-\mathrm{ft})(10 \mathrm{ft})$
$-20,000 \mathrm{k}$-ft ${ }^{3}$

$$
\begin{aligned}
& -\left(\frac{1}{6}\right)[5 \mathrm{ft}+2(10 \mathrm{ft})](150 \mathrm{k}-\mathrm{ft})(5 \mathrm{ft}) \\
& -3,125 \mathrm{k}-\mathrm{ft}^{3}
\end{aligned}
$$

## reaundant tor eacn redundant probiem.

- Calculate the flexibility coefficient associated with the relevant deflections for each redundant problem;


## Redundant Problem



## reaundant tor eacn redundant probiem.

- Calculate the flexibility coefficient associated with the relevant deflections for each redundant problem;


## Flexibility Coefficient



## Solve the Flexibility Coefficient Problem

## Real System



## Solve the Flexibility Coefficient Problem

## Virtual System <br> to measure $\delta_{D D}$



## FBD of the Flexibility Coefficient Problem


$M_{P}$ Diagram for the Flexibility Coefficient Problem

$\mathrm{M}_{\mathrm{Q}}$ Diagram for the Flexibility Coefficient Problem


## Solve the Flexibility Coefficient Problem

$$
1 \cdot \delta_{D D}=\frac{1}{E I} \int_{0}^{L} M_{Q} M_{P} d x
$$


$M_{1} M_{3} L$
$(10 \mathrm{ft})(10 \mathrm{ft})(10 \mathrm{ft})$
$1000 \mathrm{ft}^{3}$
$M_{1} 10 \mathrm{ft}$


$$
\delta_{D D}=\frac{1333 \mathrm{ft}^{3}}{E I}
$$



$$
\frac{1}{3} M_{1} M_{3} L
$$

$$
\Delta_{D D}=D_{y}\left(\frac{1333 \mathrm{ft}^{3}}{E I}\right)
$$

$$
\left(\frac{1}{3}\right)(10 \mathrm{ft})(10 \mathrm{ft})(10 \mathrm{ft})
$$

$$
333.333 \mathrm{ft}^{3}
$$

## Compatibility Equation at Point D

## Compatibility at Point D

$$
\Delta_{D}+\Delta_{D D}=0
$$

Compatibility Equation in terms of Redundant and Flexibility Coefficient

$$
\begin{gathered}
\Delta_{D}+D_{y} \delta_{D D}=0 \\
-\frac{23,125 \mathrm{k}-\mathrm{ft}^{3}}{E I}+D_{y}\left(\frac{1333 \mathrm{ft}^{3}}{E I}\right)=0
\end{gathered}
$$

Solve for $\boldsymbol{D}_{\boldsymbol{y}}$

$$
D_{y}=\frac{23,125 \mathrm{k}-\mathrm{ft}^{3}}{E I}\left(\frac{E I}{1333 \mathrm{ft}^{3}}\right) \quad D_{y}=17.34 \mathrm{k}
$$



## Moment Diagram for the Frame



> Moment diagram is drawn on the compression side of the member

## Moment Diagrams for the Primary and Redundant Problems



## Choose sign convention for internal forces for both horizontal and vertical members

## For horizontal member BDE



For vertical member ABC


